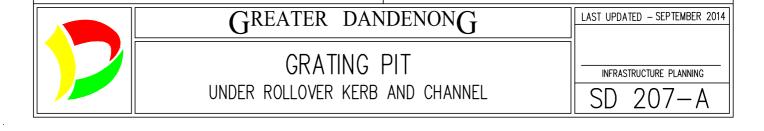
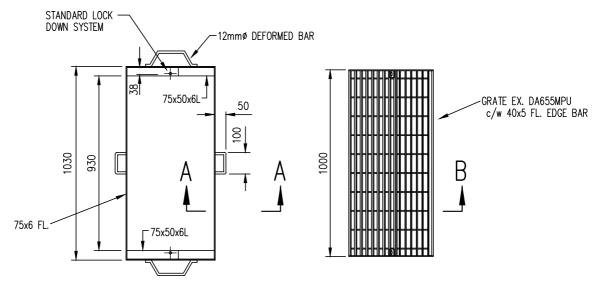


NOTE:

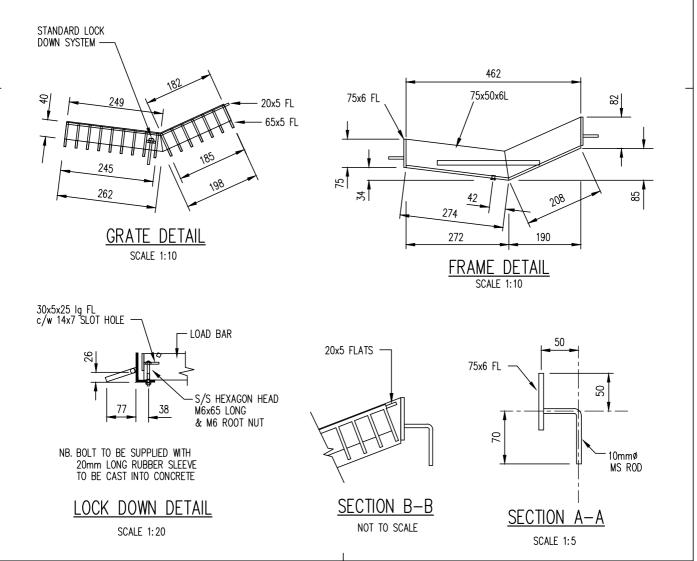
ALL MEASUREMENTS ARE IN MILLIMETRES UNLESS OTHERWISE STATED





NOTES

- 1. FINISH HOT DIPPED GALVANISED.
- 2. WEIGHT 58 KG.
- 3. ALL MEASUREMENTS ARE IN MILLIMETRES.
- 4. GRATES ARE AVAILABLE FROM WEBFORGE (VIC.) 142-146 FAIRBANKS ROAD, CLAYTON SOUTH Ph 03 8551 2414 GRATE REFERENCE MGG94HV21 (WG 21) OR APPROVED EQUIVALENT. GRATES SHALL CONFORM TO AS 3996-2006





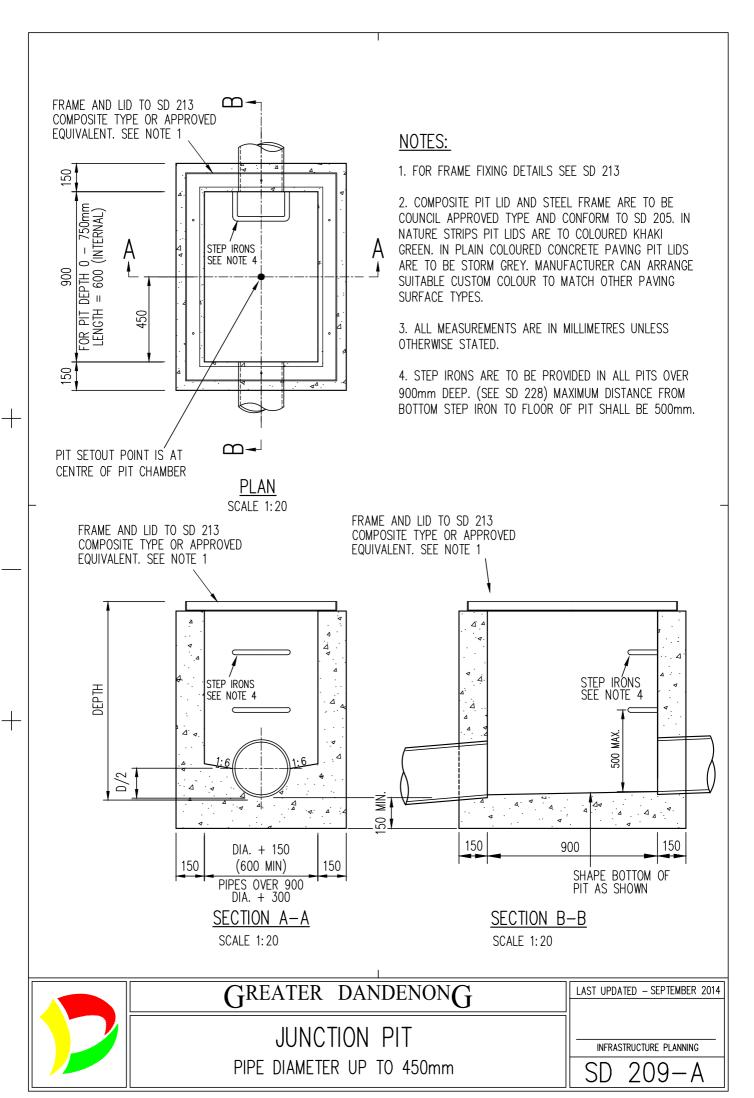
GREATER DANDENONG

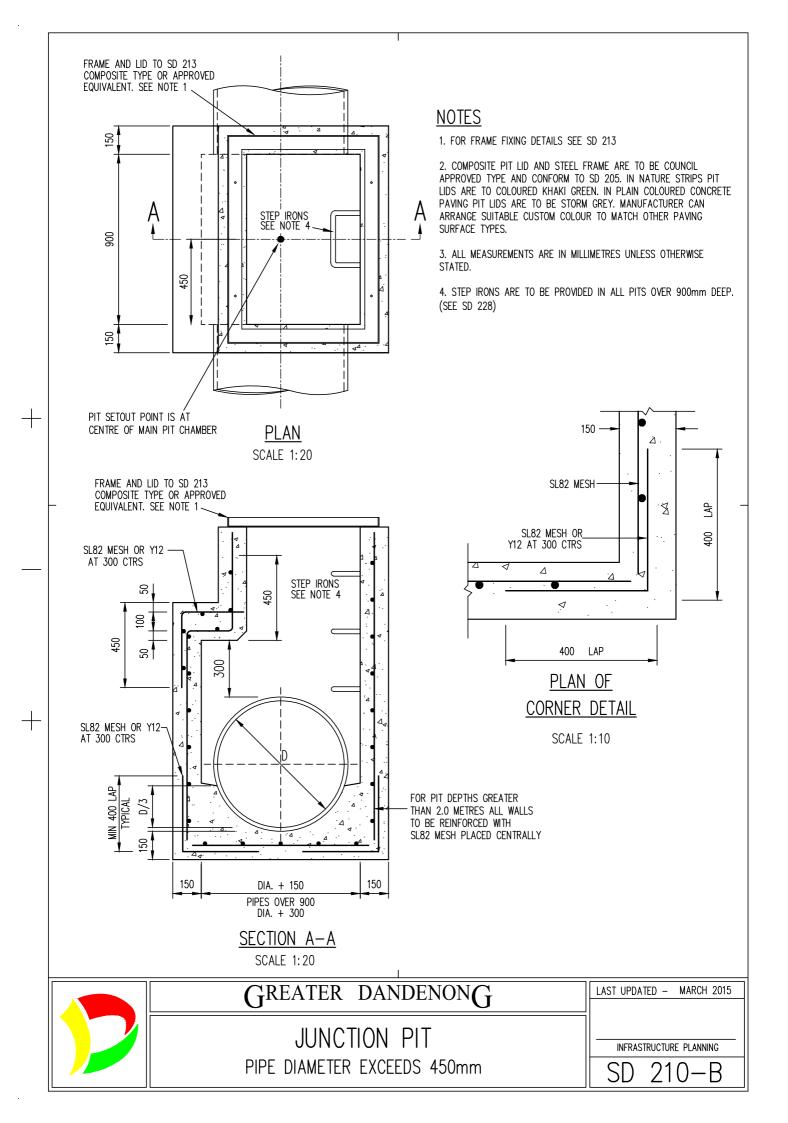
GRATING PIT — GRATE AND FRAME ROLLOVER KERB AND CHANNEL

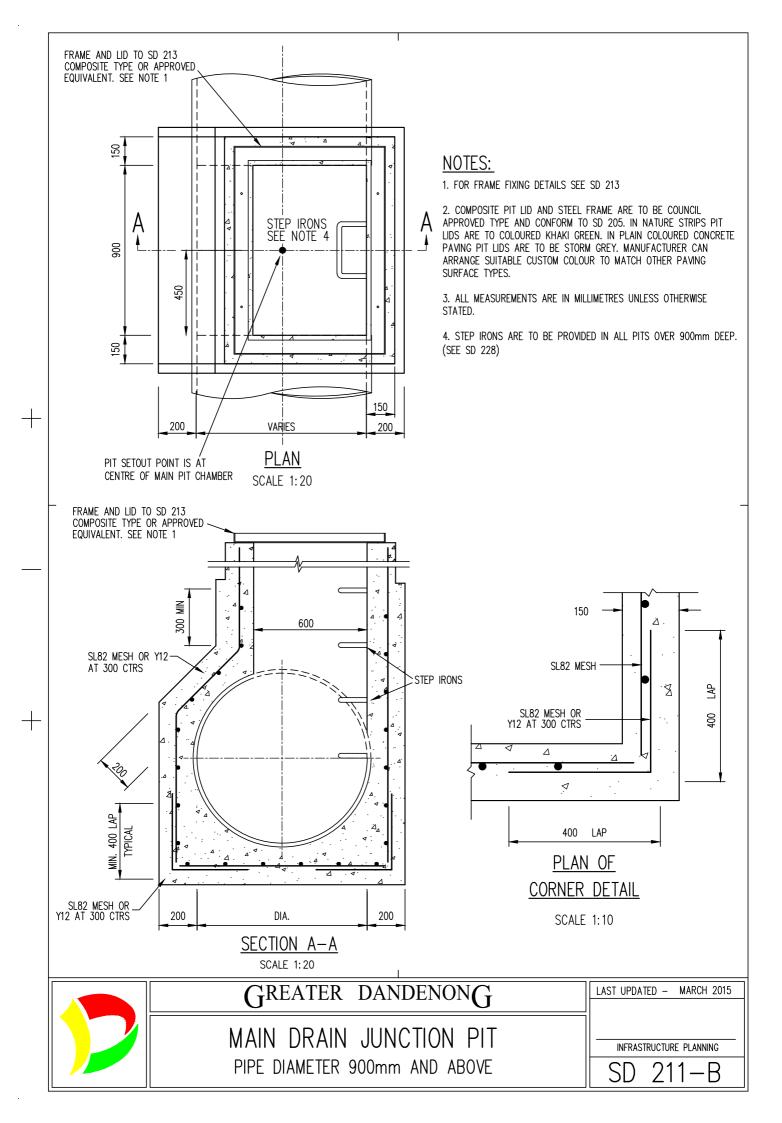
LAST UPDATED - SEPTEMBER 2014

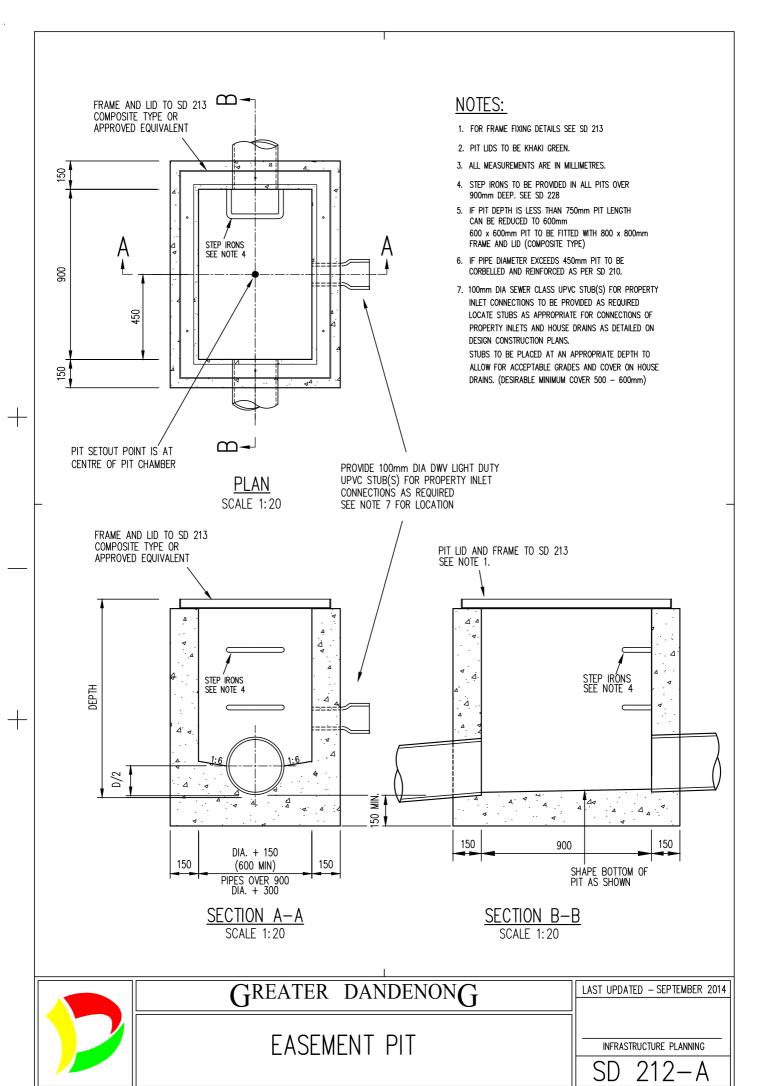
INFRASTRUCTURE PLANNING

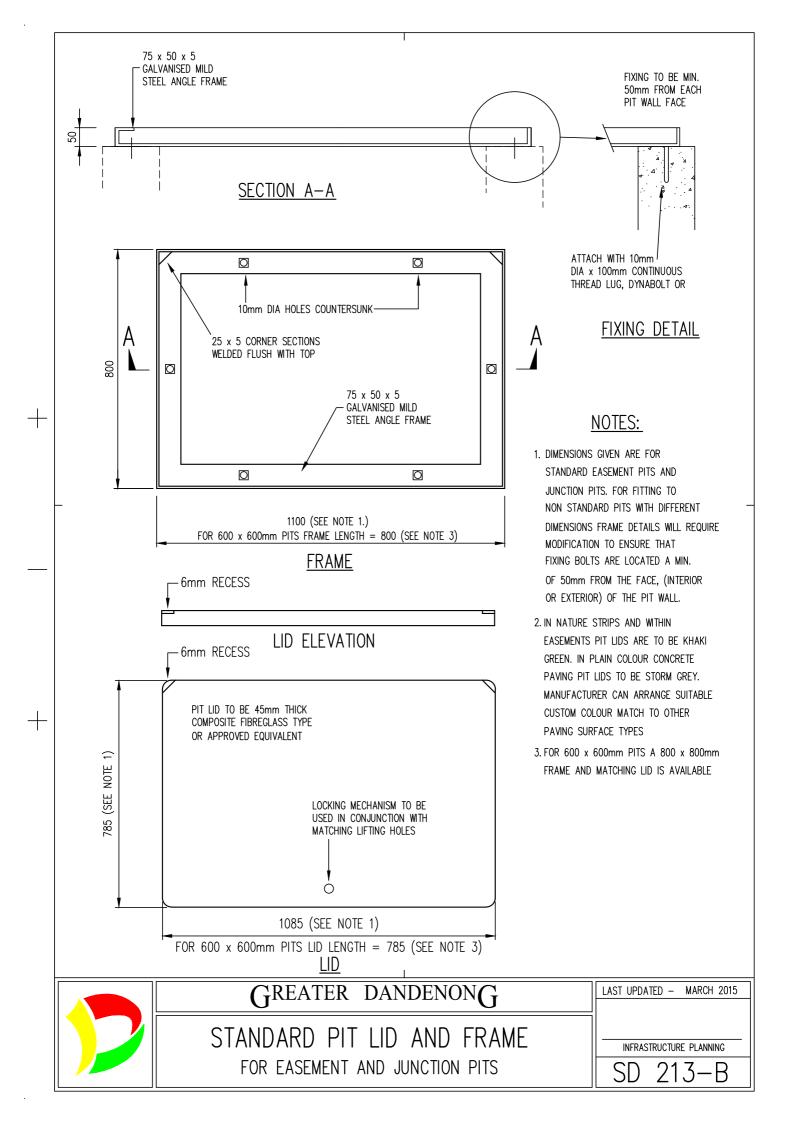
SD 208-A

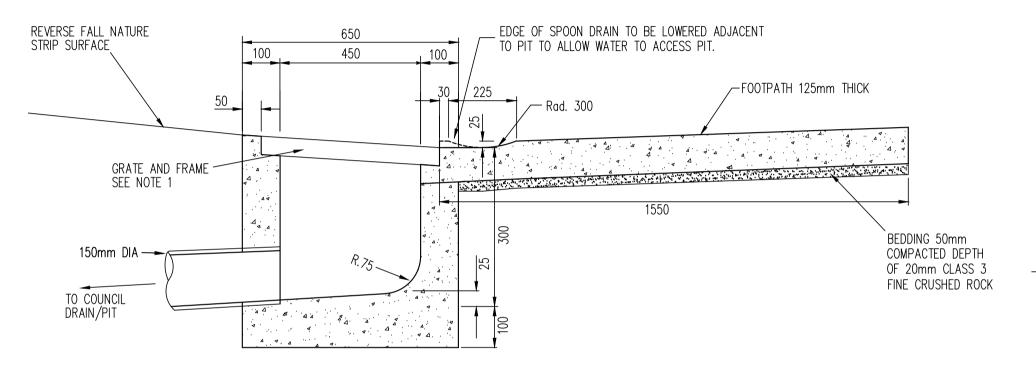












ELEVATION OF PATH, SPOON DRAIN AND PIT

NOTES:

- GALVANISED STEEL GRATE AND FRAME TO BE R&S ROAD SAFETY GRATING NO. SG-H-MSG-W4545 (OR APPROVED EQUIVALENT). GRATING SHALL BE PEDESTRIAN AND BICYCLE SAFE TO AS 3996-2008
- 2. INTERNAL DIMENSIONS OF PIT TO BE 450 x 450mm
- 3. ALL MEASUREMENTS ARE IN MILLIMETRES UNLESS OTHERWISE STATED



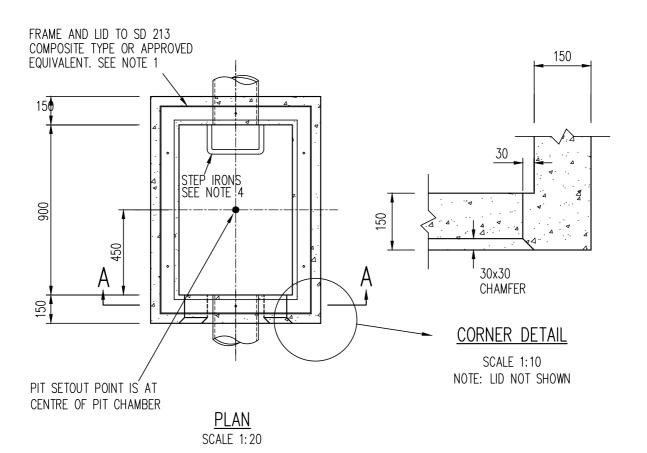
GREATER DANDENONG

FOOTPATH GRATING PIT AND SPOON DRAIN

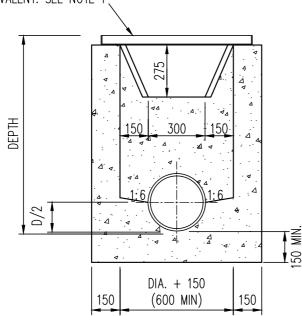
LAST UPDATED - APRIL 2015

INFRASTRUCTURE PLANNING

SD 214-B



FRAME AND LID TO SD 213 COMPOSITE TYPE OR APPROVED EQUIVALENT. SEE NOTE 1



NOTES:

- 1. FOR FRAME FIXING DETAILS SEE SD 213
- 2. COMPOSITE PIT LID AND STEEL FRAME ARE TO BE COUNCIL APPROVED TYPE AND CONFORM TO SD 205. PIT LIDS ARE TO COLOURED KHAKI GREEN.
- 3. ALL MEASUREMENTS ARE IN MILLIMETRES UNLESS OTHERWISE STATED.
- 4. STEP IRONS ARE TO BE PROVIDED IN ALL PITS OVER 900mm DEEP. (SEE SD 228) MAXIMUM DISTANCE FROM BOTTOM STEP IRON TO FLOOR OF PIT SHALL BE 500mm.

SECTION A-A
SCALE 1:20



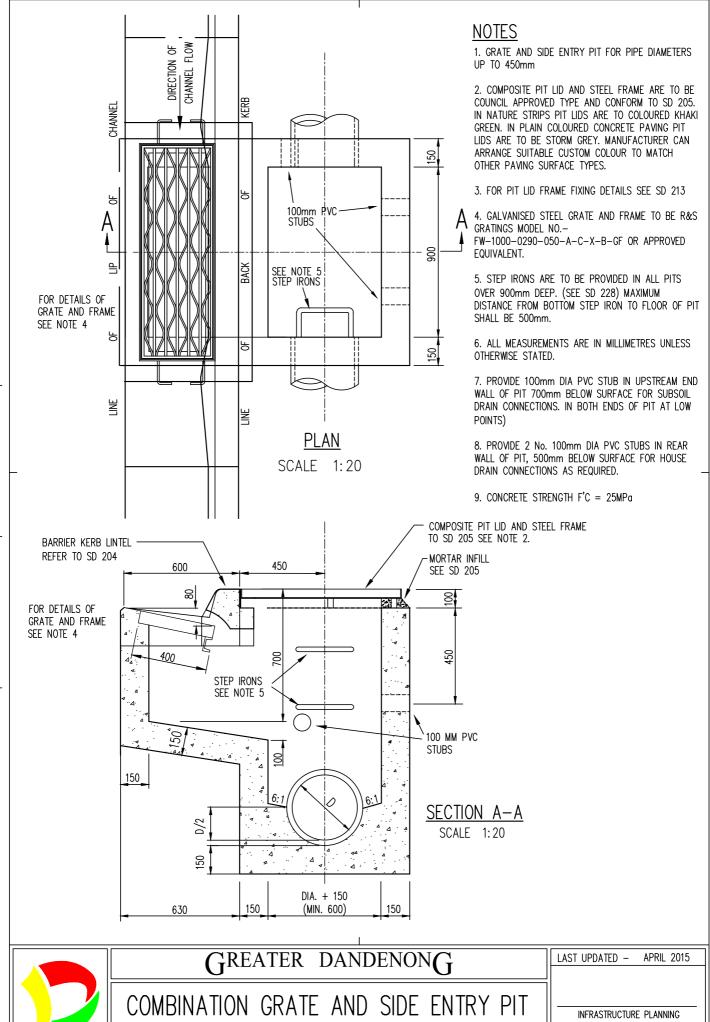
GREATER DANDENONG

ROADSIDE CATCH PIT 900 X 600mm

LAST UPDATED - SEPTEMBER 2014

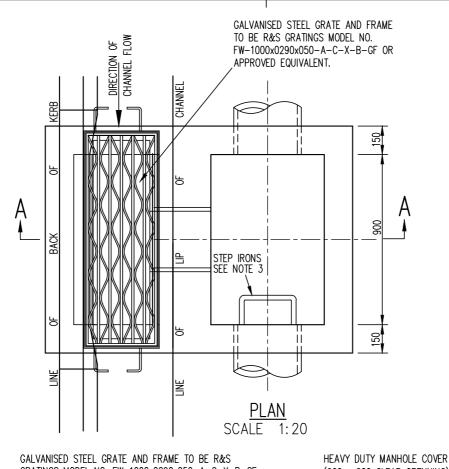
INFRASTRUCTURE PLANNING

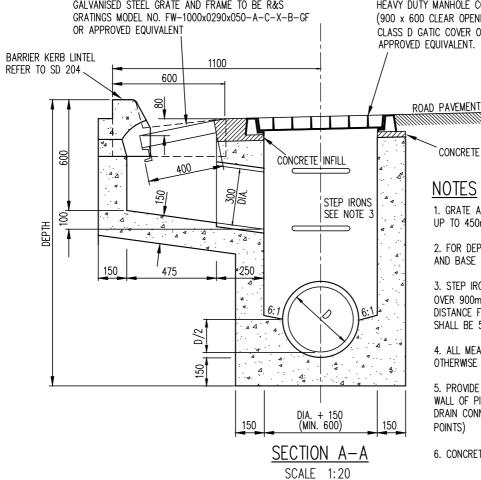
SD 215-A



FOR BARRIER TYPE KERB AND CHANNEL

216





(900 x 600 CLEAR OPENNING) CLASS D GATIC COVER OR APPROVED EQUIVALENT.

NOTES

CONCRETE INFILL

- 1. GRATE AND SIDE ENTRY PIT FOR PIPE DIAMETERS UP TO 450mm
- 2. FOR DEPTHS GREATER THAN 1.5m MINIMUM WALL AND BASE THICKNESS SHALL BE 200mm
- 3. STEP IRONS ARE TO BE PROVIDED IN ALL PITS OVER 900mm DEEP. (SEE SD 228) MAXIMUM DISTANCE FROM BOTTOM STEP IRON TO FLOOR OF PIT SHALL BE 500mm.
- 4. ALL MEASUREMENTS ARE IN MILLIMETRES UNLESS OTHERWISE STATED.
- 5. PROVIDE 100mm DIA PVC STUB IN UPSTREAM END WALL OF PIT 700mm BELOW SURFACE FOR SUBSOIL DRAIN CONNECTIONS. IN BOTH ENDS OF PIT AT LOW POINTS)
- 6. CONCRETE STRENGTH F'C = 25MPa

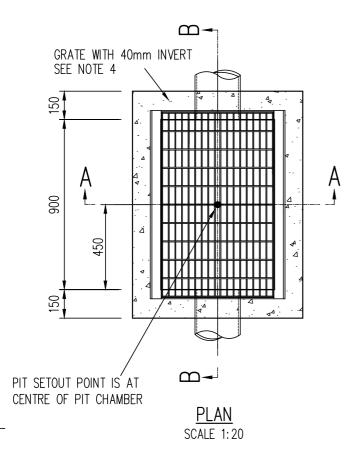


GREATER DANDENONG

COMBINATION GRATE AND SIDE ENTRY PIT FOR BARRIER TYPE KERB AND CHANNEL WHERE PIPE IS LOCATED UNDER ROAD PAVEMENT

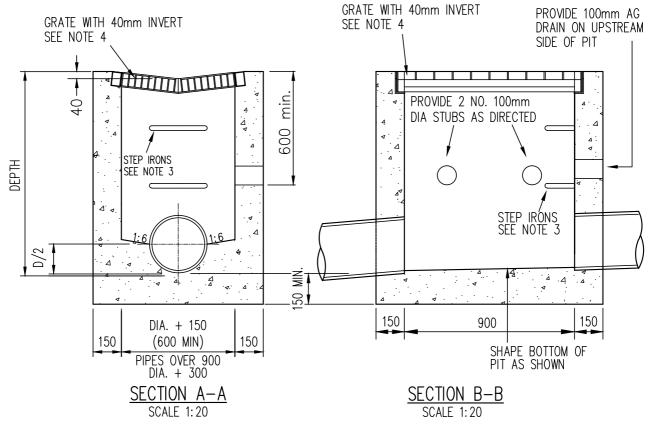
LAST UPDATED -APRIL 2015

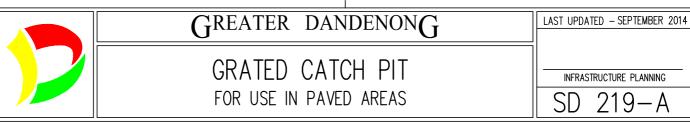
INFRASTRUCTURE PLANNING



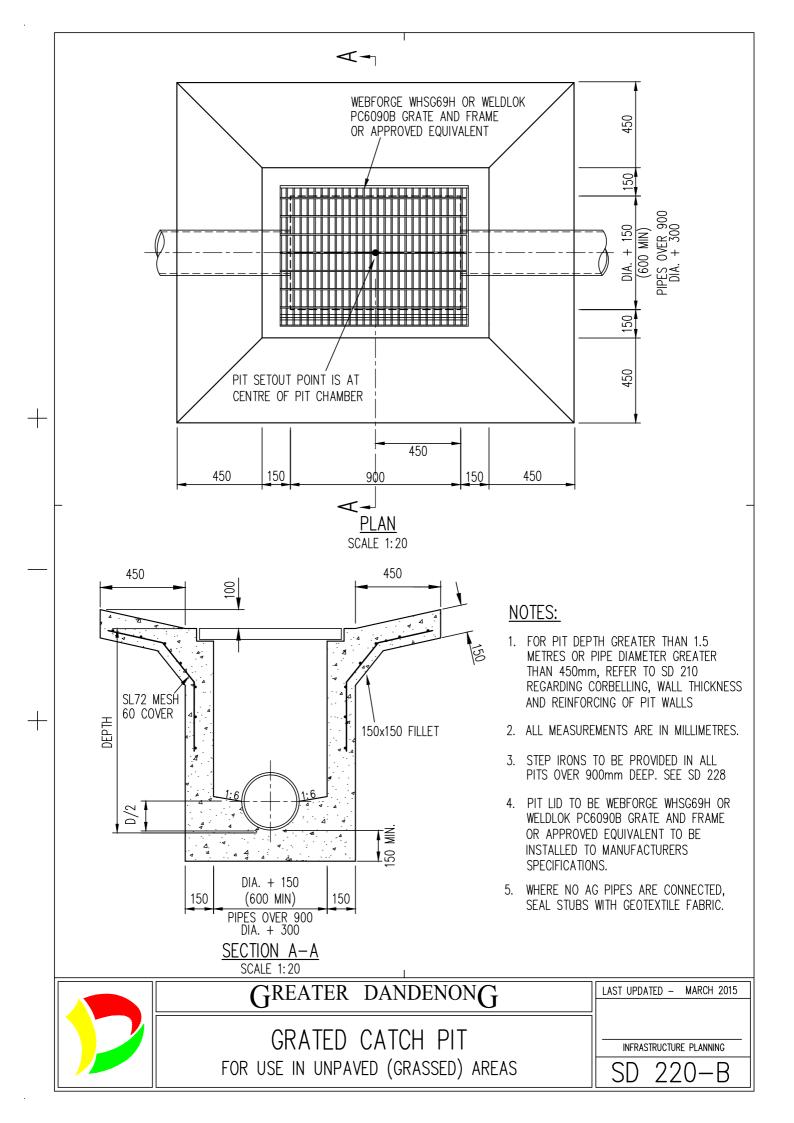
NOTES:

- 1. FOR PIT DEPTH GREATER THAN 1.5 METRES OR PIPE DIAMETER GREATER THAN 450mm REFER TO SD 210 REGARDING CORBELLING, WALL THICKNESS AND REINFORCING OF PIT WALLS
- 2. ALL MEASUREMENTS ARE IN MILLIMETRES.
- 3. STEP IRONS TO BE PROVIDED IN ALL PITS OVER 900mm DEEP. SEE SD 228
- 4. GRATE TO BE HINGED WEBFORGE MODEL MGG96HHV12 (WG-12) OR APPROVED EQUIVALENT WITH 40mm INVERT. GRATE TO BE "BICYCLE SAFE" IN ACCORDANCE WITH AUSTRALIAN STANDARD AS3996-2006
- 5. WHERE NO AG PIPES ARE CONNECTED, SEAL STUBS WITH GEOTEXTILE FABRIC.



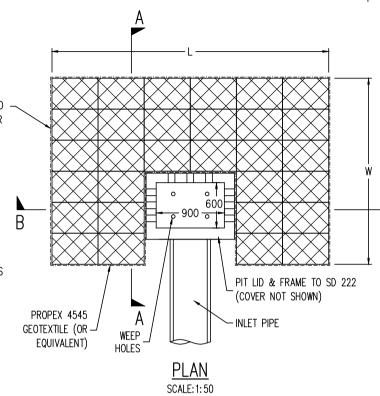


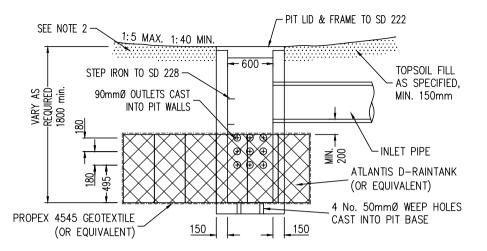
INFRASTRUCTURE PLANNING



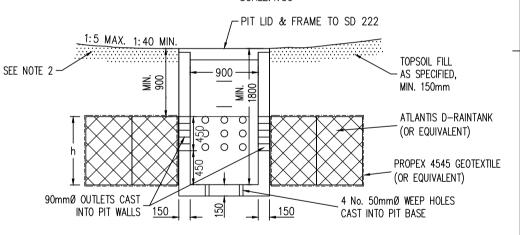
ATLANTIS D-RAINTANK (OR EQUIVALENT). TANKS TO BE INTERLOCKED TOGETHER VERTICALLY AND FULLY WRAPPED PROPEX 4545 GEOTEXTILE FILTER FABRIC (OR EQUIVALENT). TANKS TO HAVE THE FOLLOWING PERFORMANCE CRITERIA:

- TANKS TO HAVE A MINIMUM FLOW RATE OF 2000 I/min UNDER 1.0m STATIC HEAD
- ii) TANKS SOLID WALL AREA NOT TO EXCEED 25%
- iii) TANKS TO BE MADE FROM RECYCLED PLASTIC MATERIALS





SECTION A-A SCALE: 1: 50



SECTION B-B SCALE: 1: 50

NOTES:

- 1. FOR USE WITHIN AREAS WITH SANDY SOIL ONLY
- 2. SOAK AREA TO BE LOCATED IN A NATURAL OR CREATED BASIN (TURKEYS NEST). THE DISTURBED AREA BE ROLLED WITH HAND ROLLER AND SOWN WITH APPROVED GRASS SEED. CREATED BASINS MUST BE CONSTRUCTED SO THEY APPEAR NATURAL.
- 3. TEST HOLES TO THE FULL DEPTH OF THE PROPOSED SOAKAGE PIT MUST BE DUG TO DETERMINE WHETHER THE SOIL IS SUITABLE AND TEST CARRIED OUT BY A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER TO ESTABLISH THE INFILTRATION RATE OF THE SOIL BEFORE THE LOCATION AND DEPTH OF PIT IS FINALISED.
- 4. MINIMUM SOAKAGE PIT VOLUME 6.8m³
- ALL CONCRETE USED ON PIT TO BE 32MPa AND VIBRATED INTO POSITION AS PER AS3600, SECTION 19.
- 6. ALL MEASUREMENTS IN MILLIMETRES.
- 7. PIT LID AND FRAME TO SD 222
- 8. STEP IRONS TO SD 228
- 9. SEE SHEET 2 FOR SOAKAGE PIT SIZING DESIGN GUIDELINES.



GREATER DANDENONG

SOAKAGE PIT SHEET 1 OF 2 SHEETS

LAST UPDATED — SEPTEMBER 2014

INFRASTRUCTURE PLANNING

SD 221-1-A

SOAKAGE PIT SIZING DESIGN GUIDELINES:

DESIGN CRITERIA:

- SOAK PITS TO BE USED ONLY WHERE CATCHMENT HAS NOT POSITIVE OUTFALL
- THE TOTAL VOLUME OF RUNOFF FROM THE CATCHMENT AREA TO THE SOAKAGE PIT SHOULD BE CALCULATED USING A 1 IN 5 YEAR 12 HOUR DURATION ARI DESIGN STORM.
- RUNOFF VOLUME TO BE DETERMINED BY APPLYING COMBINED EQUIVALENT CATCHMENT AREA WITH DESIGN RAINFALL DEPTH (SEE TABLE BELOW).
- THE ENGINEER SHALL DETERMINE APPROPRIATE PROPORTIONS CONTRIBUTING IMPERVIOUS AND PERVIOUS AREAS.
- THE SOAKAGE PIT IS REQUIRED TO EMPTY FROM FULL LESS THAN 50% VOLUME WITHIN 24 HOURS OF THE STORM EVENT.
- NO SURFACE WATTER IS TO RESULT FROM A 1 IN 5 YEAR 12 HOUR ARI STORM EVENT (IE. CONTAINED WITHIN SOAKAGE PIT).
- MAXIMUM ROADWAY FLOODING IS 150mm DEPTH AND IS TO BE CONTAINED WITHIN THE ROAD KERB AND CHANNEL FOR A 1 IN 10 YEAR 24 HOUR ARI STORM EVENT.
- NO ENCROACHMENT WITHIN 300mm OF ANY HABITABLE OR NON HABITABLE FLOOR LEVEL RESULTING FROM THE INUNDATION BY A 1 IN 100 YEAR 24 HOUR ARI STORM EVENT.

<u>EQUATIONS:</u> $I = (Equiv. A_1 + Equiv. A_p)R$, $O = a_{50} fD$, S = I - O, $t_{50} = \frac{0.5S}{a_{50} f}$

WHERE: I = INFLOW DRAINED TO SOAKAGE PIT FROM STORM EVENT (m3),

 $A_1 = IMPERVIOUS AREA (m^2),$

 $A_P = PERVIOUS AREA (m^2),$

R = DESIGN RAINFALL DEPTH OBTAINED FROM THE TABLE BELOW (m),

= OUTFLOW FROM INFILTRATION INTO THE SOIL DURING STORM EVENT (m3),

 $a_{50} = 50\%$ OF TOTAL AVAILABLE INTERNAL WALL AREA (m²),

L & W = LENGTH AND WIDTH OF OVERALL SOAKAGE PIT (m),

h = OVERALL HEIGHT OF SOAKAGE PIT (m),

f = SOIL INFILTRATION RATE (m/s),

D = STORM DURATION (sec),

- STORAGE VOLLIME OF SOAKAGE PLT AND THE

S = STORAGE VOLUME OF SOAKAGE PIT AND TURKEYS NEST (IF REQUIRED). (m³),

t $_{50}$ = TIME FOR SOAKAGE PIT TO DRAIN TO 50% VOLUME (sec).

DESIGN RAINFALL DEPTH:

ARI	DESIGN RAINFALL DEPTH (mm)
1 IN 5	53.14
1 IN 10	60.54
1 IN 100	96.58

EXAMPLE CALCULATION:

DESIGN DATA:

- ASSUME THAT THE TOTAL CATCHMENT IS 0.34 Ha $(3,400 \text{ m}^2)$, THAT THE IMPERVIOUS AREA IS 0.09 Ha (900 m^2) AND THE PERVIOUS AREA IS 0.25 Ha $(2,500 \text{ m}^2)$.
- ASSUME THAT 90% OF IMPERVIOUS AND 30% PERVIOUS AREA CONTRIBUTE AS RUNOFF.
- ASSUME OVERALL HEIGHT OF SOAKAGE PIT (h) TO BE 0.903 m.
- USING A 1 IN 5 YEAR 12 HOUR ARI DESIGN STORM, THE RAINFALL DEPTH (R) IS 53.14 mm.
- FROM A SOIL TEST THE INFILTRATION RATE (f) WAS DETERMINED TO BE 1.15×10^{-4} m/s.

INFLOW VOLUME TO SOAKAGE PIT CALCULATION:

$$I = (Equiv. A_1 + Equiv. A_P)R$$

 $I = (900 \times 0.9 + 2,500 \times 0.3) \times 0.05314 = 82.90 \text{ m}^3$

SOAKAGE PIT SIZING:

OUTFLOW FROM SOAKAGE PIT:

O =
$$\alpha_{50}$$
 fD & α_{50} = 2 x [(L x 0.5h) + (W x 0.5h)] = 1.806L (ASSUMING L = W) O = 1.806L x 0.000115 x (12 x 60 x 60) = $8.972L$

STORAGE VOLUME OF SOAKAGE PIT:

S = L x W x
$$0.903 = 0.903L^2$$
 (ASSUMING L = W) & S = I - 0, THEREFORE $0.903L^2 + 8.972L - 82.90 = 0$, SOLVING, L = 5.825 m & S = 30.637 m³

THEREFORE ADOPT:

$$L = \underline{6.0 \text{ m}}$$
 & $W = \underline{5.7 \text{ m}}$,
ACTUAL S = 0.903 x 6.0 x 5.7 = 30.88 m³

CHECK TIME FOR EMPTYING:

ACTUAL
$$a_{50} = 2 \times [(6.0 \times 0.5 \times 0.903) + (5.7 \times 0.5 \times 0.903)] = \frac{10.565 \text{m}^2}{10.565 \text{m}^2}$$

$$t_{50} = \frac{0.5 \text{S}}{10.565 \times 30.88} = 12709 \text{ sec (JUST UNDER 3 HOURS \& 32 MINUTES)}$$

LESS THAN 24 HOURS, THEREFORE OK

NOTES:

- FOR USE WITHIN THE CITY OF GREATER DANDENONG ONLY. THE ENGINEER IS RESPONSIBLE FOR THE SIZING OF THE SOAKAGE PIT AND WHILST THESE GUIDELINES REPRESENT BEST PRACTICE, THE ENGINEER SHOULD TAKE INTO CONSIDERATION THE SPECIFIC LOCAL SITE CONDITIONS WHEN SIZING THE SOAKAGE PIT.
- 2. SEE SHEET 1 FOR SOAKAGE PIT INSTALLATION DETAILS.



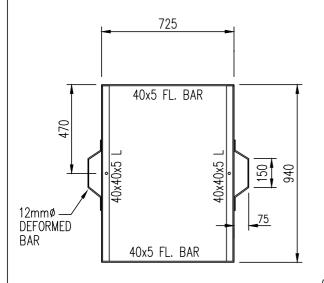
GREATER DANDENONG

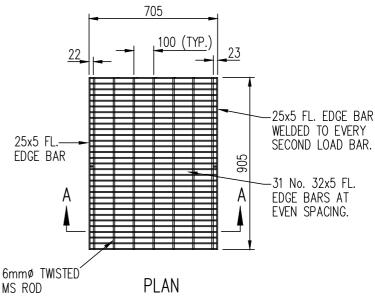
SOAKAGE PIT SHEET 2 OF 2 SHEETS

LAST UPDATED - SEPTEMBER 2014

INFRASTRUCTURE PLANNING

|SD 221-2-A



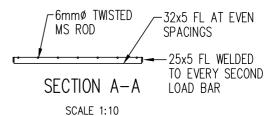


PLAN

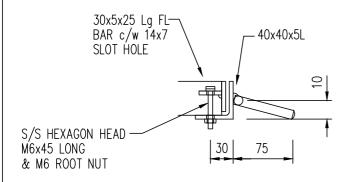
ELEVATION

FRAME DETAIL

SCALE 1:20



GRATE DETAIL SCALE 1:20



NB: BOLT TO BE SUPPLIED WITH 20mm LONG RUBBER SLEEVE TO BE CAST INTO CONCRETE.

LOCK DOWN DETAIL

SCALE 1:5

NOTES

- 1. FINISH HOT DIPPED GALVANISED.
- 2. WEIGHT 40KG.
- 3. ALL MEASUREMENTS ARE IN MILLIMETRES.
- 4. GRATES ARE AVAILABLE FROM WEBFORGE (VIC) 142-146 FAIRBANKS ROAD, CLAYTON SOUTH Ph. 03 8551 2414 GRATE REFERENCE - MSG69B



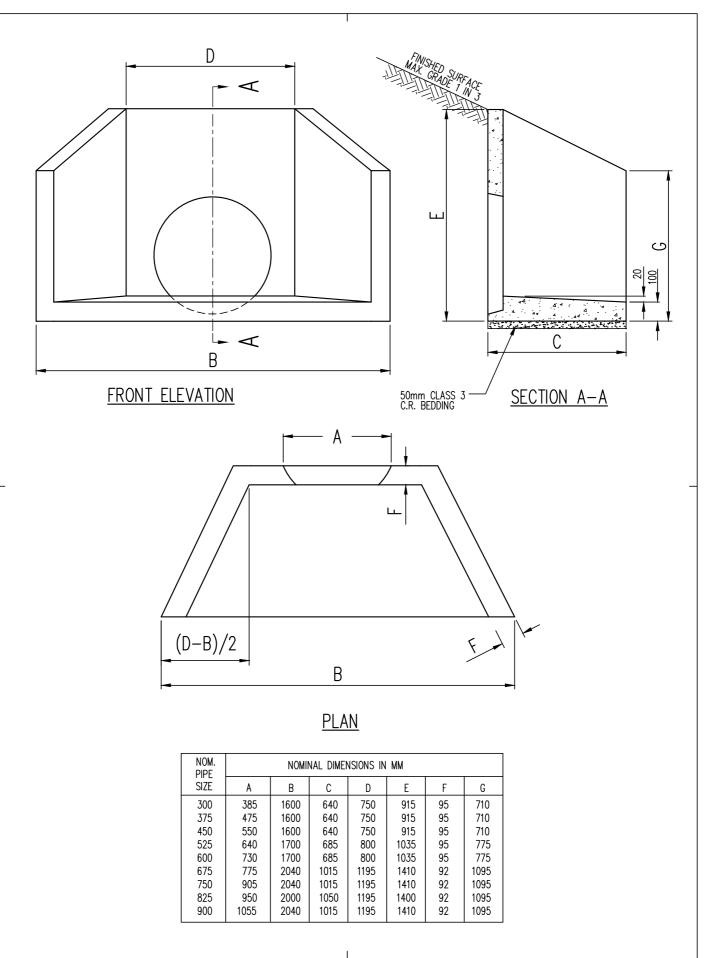
GREATER DANDENONG

LIGHT DUTY STEEL GRATE AND FRAME FOR SOAKAGE PIT

LAST UPDATED - SEPTEMBER 2014

INFRASTRUCTURE PLANNING

SD 222-A





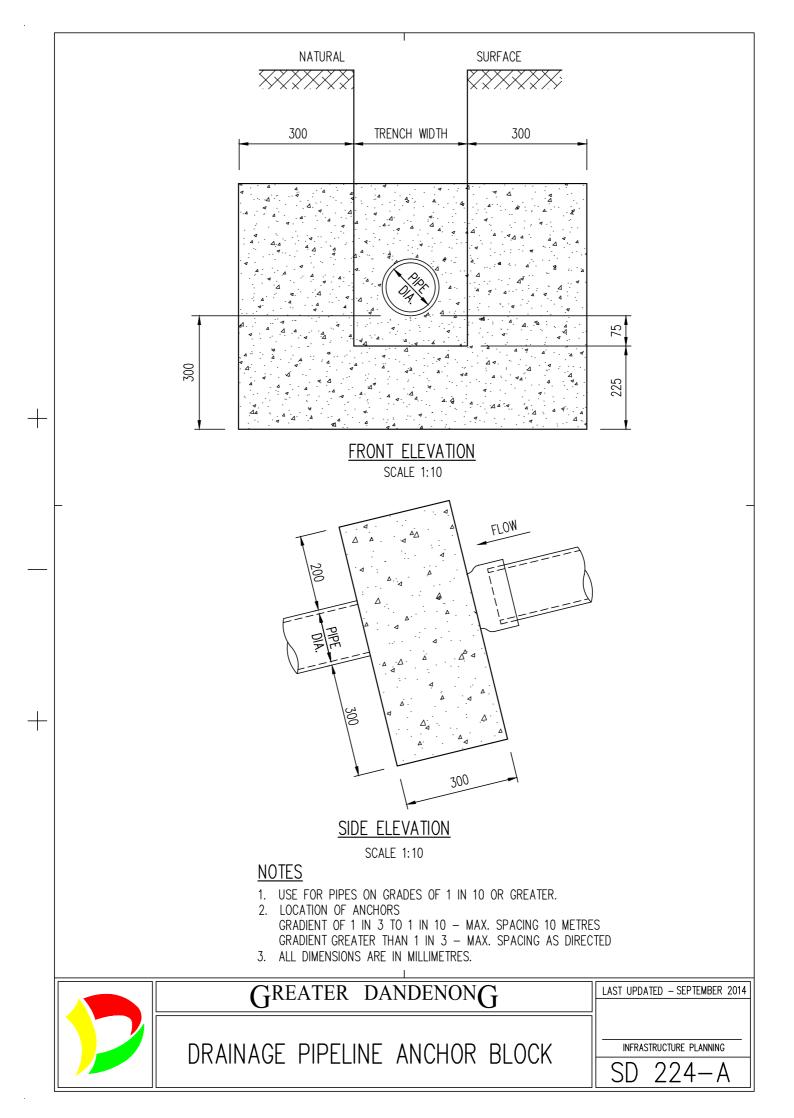
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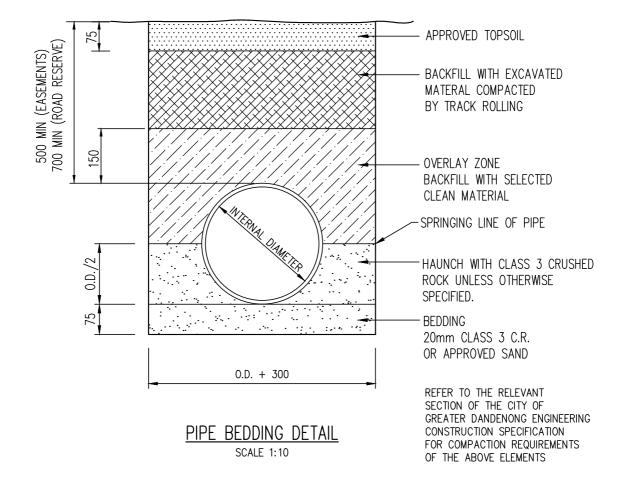
PRECAST CONCRETE ENDWALL

LAST UPDATED — SEPTEMBER 2014

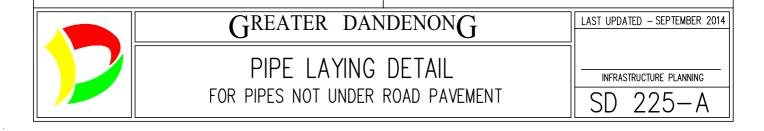
INFRASTRUCTURE PLANNING

SD 223-A

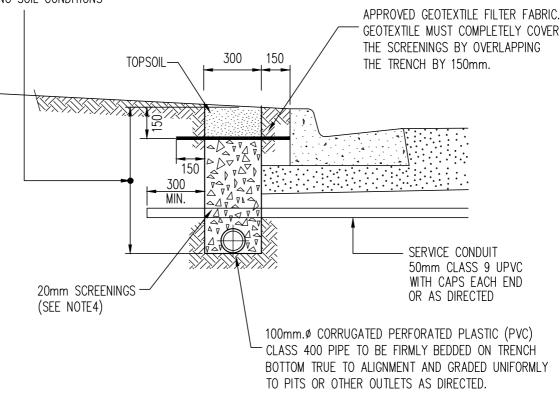




- 1. ALL TRENCHES UNDER KERB AND CHANNEL, DRIVEWAYS, FOOTPATHS AND ROAD PAVEMENT SHOULD BE BACKFILLED AS PER STANDARD SPECIFICATION.
- 2. ALL 150 PIPES TO BE PVC (SH), 225 TO BE PVC (SH) OR RRJ R.C./FRC, ALL PIPE SIZES TO BE RUBBER RING JOINTED (RRJ)
- 3. ALL R.C./ F.R.C. PIPES TO BE CLASS 2 UNLESS SPECIFIED DIFFERENTLY.
- 4. EASEMENT DRAINS TO BE LOCATED TO ENSURE 500mm MINIMUM FROM EDGE OF PIPE TO EDGE OF EASEMENT. REINSTATEMENT WORKS REFER TO NOTE 1.
- 5. IF TRENCH IS WITHIN 150mm OF KERB THEN TRENCH IS TO BE BACKFILLED WITH SELECT BACKFILL FROM SITE AS DIRECTED. FOR NEW WORKS OR REINSTATEMENT WORKS REFER TO NOTE 1.
- 6. MINIMUM PIPE REQUIREMENTS UNDER ROAD PAVEMENTS AND TAKING ROAD RUNOFF TO BE 300mm DIA. RRJ R.C.



DEPTH AS DETERMINED BY
APPROPRIATELY QUALIFIED SOILS
ENGINEER, HAVING REGARD FOR
PROPOSED DESIGN PAVEMENT
AND EXISTING SOIL CONDITIONS

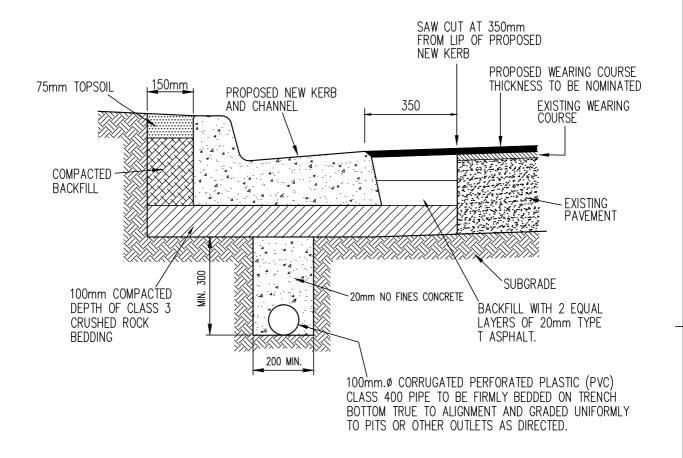


LOCATION OF SUBSOIL DRAINAGE PIPE

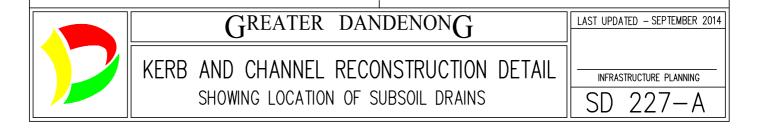
SCALE 1:20

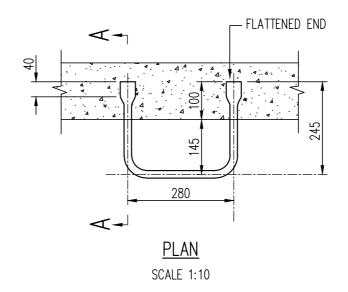
- 1. DEFLECT TRENCH OVER LAST 2.0m TO CONNECT TO PITS
- 2. A.G. DRAIN TO BE A MINIMUM OF 25mm CLEAR BELOW SERVICE CONDUITS
- 3. A.G. DRAINS TO BE LOCATED BEHIND KERB AND CHANNEL
- 4. BACKFILL TO BE:
 - (i) 20mm SCREENINGS
 - (ii) 20mm CLEAN SCORIA
 - (iii) 20mm RECYCLED CONCRETE WITH NO FINES
 - (iv) 20mm NO FINES CONCRETE

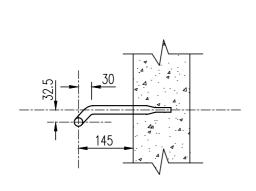


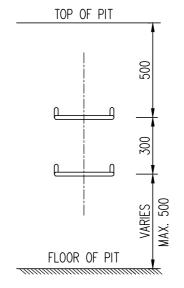


- 1. DEFLECT TRENCH OVER LAST 2.0m TO CONNECT TO PITS
- 2. SUBSOIL DRAIN TO BE A MINIMUM OF 25mm CLEAR BELOW SERVICE CONDUITS ON BOTH SIDES OF ROAD
- 3. SUBSOIL DRAINS TO BE LOCATED UNDER KERB AND CHANNEL









SECTION A-A

SCALE 1:10

ELEVATION

SCALE 1:20

- 1. STEP IRONS FROM 20mmø STEEL BAR, HOT DIP GALVANIZED
- 2. PROVIDE STEPIRONS IN ALL PITS OVER 900mm DEPTH.
- 3. STEPIRONS TO BE LOCATED IN PIT WALLS, CLEAR OF PIPES WHERE POSSIBLE
- 4. MEASUREMENTS ARE IN MILLIMETRES.

